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Member of the A.I.A.

CONSTRUCTION DETAILS WORKBOOK

PROJECT:

Lot 5 Creekside Ranch
Mount Vernon, Washington

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N/A

N/A

Structural Notes

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1. GENERAL:

- 1.1 All construction shall be in accordance with the minimum provisions of the 2009 Edition of the International Building Code; where these plans and specifications do not state specifically otherwise the provisions of the IBC shall apply.
- 1.2 Typical details and schedules in the Project Manual shall be used wherever applicable.
- 1.3 The SubContractors shall verify all dimensions in the field, and upon discovery of any discrepancies shall be immediately reported to John R. Cox & Associates. Do not scale drawings.
- 1.4 SubContractors shall verify all 'fit' conditions in the field. Should the Subcontractor or fabricator note any conflicts or errors in the Plans and/or specifications, they shall be brought to the immediate attention of John R. Cox & Associates. If any questions arise during construction pertaining to any structural matter, John R. Cox & Associates shall be consulted immediately for prompt resolution.
- 1.5 The Subcontractor is responsible for all erection and/or temporary bracing and shoring. Where the floor is used to brace the walls, do not backfill retaining walls until main floor plywood is in place.
- 1.6 Loads (Unless Noted Otherwise Elsewhere):
 - 25 psf Roof Live Load
 - 40 psf Residential Floor Live Load
 - 60 psf Residential Deck Live Load
 - 100 psf Stairway Live Load
 - 80 mph Basic Wind Speed, Exposure "C"
 - Zone 3 IBC Seismic Zone Category

2. Foundations:

- 2.1 Footings are designed for a minimum allowable soil pressure of 1500 psf. Footings shall be poured on firm, undisturbed existing earth, unless otherwise shown in plans or calculations.
- 2.2 All organic and deleterious material beneath the footings, foundations and slabs shall be removed and re-placed with granular fill compacted to approx. 95% Relative Compaction (RC). Bottom of footings to be below locally prescribed frost zone, not less than 12" for one story, not less than 18" for 2-stories. Replace all over-excavated areas with granular material compacted to approx. 95% RC.

- 2.3 Pads and footings shall be minimum of 8" thick unless otherwise specified. Reinforced with #4 rebar @9" O.C. each way, installed 3" clean from the ground, with no less than 2 rows in each direction.
- 2.4 Continuous footings shall be a minimum of 8" thick unless otherwise specified. Width of footing shall be as shown in the drawing, not to exceed 24" U.N.O. #4 rebar continuous at the footing bottom.
- 2.6 Footing drains, with washed drain rock extending to within one foot of top of finished grade, shall be provided at the base of all footings and retaining walls which will have earth placed against them. Footing drains shall be 4" perforated pipe routed downgradient to daylight, unless otherwise specified. The invert elevation of all footing drains shall be lower than the bottom of adjacent footings drained.
- 2.7 Concrete slabs on grade to be 4" thick TYP. If the soil is not adequate, one (or both) of the following options should be implemented:
- A) Reinforcement shall be WWF, 6" x 6" x 10ga. or #3 at 24" OC each way, centered in a 4" slab.
 - B) Prior to installation of slab, place 4" compacted drainage fill and 2" of clean washed sand over vapor barrier.
- 2.8 Anchor bolts set in concrete or masonry shall be ASTM A307 steel, and shall have a 3-diameter hook at the embedded end (7-diameter at hold-downs). Embedment shall be 8-diameters but not less than 7 inches.

3. Wood Construction:

- 3.1 All sawn framing lumber shall be Douglas Fir #2 or better, unless otherwise shown. Provide studs directly underneath all top plate splice locations. Connect all wood members per the IBC.
- 3.2 Anchor bolts to mud sill, use 5/8" diameter x 7" embedment at 48" OC, with standard steel plate washers, wrench tight, unless otherwise shown.
- 3.3 Wood ledgers (2x8 P.T. min.) to concrete or masonry, use 5/8" diameter anchor bolts with 6-inch minimum embedment spaced 16 inches on center, staggered, unless otherwise shown.
- 3.4 Wood 2x ledgers to studs or other wood, use 16d at 4 inches on center to continuous member, or 3, 16d per stud, studs spaced 16" OC or less, unless otherwise shown.

- 3.5 Built-up beams consisting of dimension lumber (typically 2x stock) are permitted in lieu of sawn solid beams only if the 2x's are oriented such that they are not stacked on top of each other with the sum of their weak axes resisting load, but are nailed together side-by-side, with the sum of their strong axes resisting load. Use 16d face nails at 6" OC staggered into all tributary members.
- 3.6 Use pressure treated lumber in contact with concrete. Pressure treating chemicals shall be inert to and not reactive with metal and/or connectors.
- 3.7 Provide bridging or blocking at 8' OC max. in joist or rafters without continuous diaphragm support on the top and bottom (i.e. plywood on the top and gyp. on the bottom). Provide solid blocking at all bearing points, and double joists under all partition walls parallel to the floor joists. Framed floors which support posts shall be solidly blocked within the floor to positively transfer posts loads through the floor to the supports beneath.
- 3.8 Unless otherwise specified: Use washers on all bolts; tighten all bolts to a very snug wrench tight condition; use standard A307 bolts; use hooks on all embedded anchor bolts. Provide the following minimum edge distances between centerline of bolt and all edges of bolted wood, and between centerline of multiple bolts:
- 1/2 inch diameter bolts...2.5 inches
 - 5/8 inch diameter bolts...3 inches
 - 3/4 inch diameter bolts...3.75 inches
 - 7/8 inch diameter bolts...4.3 inches
 - 1 inch diameter bolts...4.75 inches
 - 1-1/4 inch diameter bolts...5 inches
- 3.9 Simpson brand is specified, however any other nationally recognized brand (Silver, KC, etc.) may be used provided that they are equivalent in their ability to carry all applied loads in all orientations.
- 3.10 The Subcontractor shall install all prefabricated items in strict accordance with the manufacturer's recommendations and requirements.
- 3.11 Where holdowns are shown on the plans, the factory specified anchor bolts, lags, or nails, which connect to the vertical member shall be installed per manufacture recommendations and/or specifications. Vertical members shall be double 2x, or single 4x material unless otherwise specified. Anchor bolts, which are too long to fit in the footing in a vertical orientation, may be bent in a smooth curve to a maximum of 90 degrees and extended horizontally within the footing. 'All-thread' with head and washers at the embedded end may be substituted for long anchor bolts.
- 3.12 The Contractor shall verify with the prefab. wood manufacturer that the specified connectors will work as intended with their product.
- 3.13 Unless otherwise shown, the following metal plate connectors shall be used:

Post caps...PC series
 Post bases...PB series
 Beam hangers...Top flange type, HB, JB, PF, HUSTF, HUTF, or equal.
 Joist hangers...LUS series, LSU for sloped, LSSU for sloped / skewed.
 Truss and rafter holdowns...H1 at each heel to double plate.
 Holdowns...STHD14 (RJ)

- 3.14 Top of retaining wall (concrete, masonry, Insteel) to floor joist: for wall perpendicular to joist, use L50, sill plates to each joist. for wall parallel to joist use 2x blocking spaced at 24"OC along wall; install perpendicular to retaining wall, between first 5 joists from wall, connect each block at sill plate with one L50.
- 3.15 For sheathing use A.P.A. trademark CDX plywood or OSB unless otherwise noted. Store and install in accordance with the recommendations of A.P.A and IBC for shear resisting vertical and horizontal diaphragms.
- 3.16 Oriented strand board, with shear resistance values similar to 1/2" plywood may be substituted for plywood on shear walls and on roof, unless otherwise specified on the Plans. If OSB is used, the same nailing and blocking schedule as per plywood shall be adhered to. Where used on roof, Contractor shall ensure that OSB meets or exceeds the proper span rating for trusses and / or rafters as installed. All OSB shall be stored and installed in accordance with manufacturer's recommendations.
- 3.17 All plywood on shear walls shall have all edges blocked. All blocking to receive edge nailing. If not otherwise specified on the Plans, standard shear wall construction shall consist of 1/2" plywood or 7/16" OSB, nailed with 8d at 6" on edges, and 12" in field. All shear walls shall be positively connected to horizontal diaphragms at their tops and bottoms per the above, or as called out in the Plans.
- 3.18 If roof diaphragm is not specified in Plans or Calculations, use 1/2" over non-blocked supports at 24" OC, Use IBC Case 1 pattern. Nail with 10d at 6" on edges, and 12" in field. Contractor to verify all span ratings.
- 3.19 All wood floor diaphragms shall be glued and nailed. Use thickness as shown on the Plans. Contractor to verify all span ratings of plywood. Where otherwise not shown on Plans, nail floor diaphragm using 8 penny (screw type nails recommended) at 6" on edges, 12" in field, non-blocked, per IBC Case 1 pattern.
- 3.20 Alternate Braced Wall Panels Schedule:
- | | |
|------|---------------------------|
| BWP | Braced Wall Panel |
| BWP3 | BWP Construction Method 3 |
| BWP5 | BWP Construction Method 5 |

ABWP	Alternate Braced Wall Panels
ABWP1	ABWP Item 1
ABWP2	ABWP Item 2

3.21 Materials for wood construction shall be as follows, except as noted otherwise.

Posts and Beams:	Douglas Fir #2 or Better
Glued-Laminated Beams (GLB):	Combination Symbol 24F, V4
Parallel Strand Lumber (PSL):	2.0E, or 2.1E, DR as shown
Laminated Veneer Lumber (LVL):	1.8E, DF
TimberStrand (LSL) Headers:	1.5E
Studs, Sills, and Plates:	Douglas Fir standard grade
Sawns Joists, Blocking, and Bridging:	Douglas Fir #2 or Better
Subflooring:	3/4 inch minimum CDX, I.D. 42/20 Glue at all supports per APA standards.
Roof Sheathing:	5/8 inch minimum CDX, I.D. 42/20
Wall Sheathing:	1/2 inch minimum CDX, I.D. 24/0
Floor Joist Insulation	R-19 U.N.O.
Stud Wall Insulation	R-19 U.N.O.
Blown Ceiling Insulation	R-32 U.N.O.

Typical Wood Beam Schedule (1 of 2)

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ID	DESCRIPTION	ACTUAL DIMENSION	GRADE/ SPECIES	CONNECTORS
B4X8	4X8 DOUGLAS FIR #2 BEAM	3 1/2" X 7 1/4"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
B4X10	4X10 DOUGLAS FIR #2 BEAM	3 1/2" X 9 1/4"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
B4X12	4X12 DOUGLAS FIR #2 BEAM	3 1/2" X 11 1/4"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
B6X8	6X8 DOUGLAS FIR #2 BEAM	5 1/2" X 7 1/4"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
B6X10	6X10 DOUGLAS FIR #2 BEAM	5 1/2" X 9 1/4"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
B6X12	6X12 DOUGLAS FIR #2 BEAM	5 1/2" X 11 1/4"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
BGLU1*	3 1/2X9 1/2 24F-V4 DOUGLAS FIR GLU-LAM BEAM	3 1/2" X 9 1/2"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BGLU2*	3 1/2X11 7/8 24F-V4 DOUGLAS FIR GLU-LAM BEAM	3 1/2" X 11 7/8"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BGLU3*	5 7/16X9 1/2 24F-V4 DOUGLAS FIR GLU-LAM BEAM	5 7/16" X 9 1/2"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BGLU4*	5 7/16X11 7/8 24F-V4 DOUGLAS FIR GLU-LAM BEAM	5 7/16" X 11 7/8"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BGLU5*	5 7/16X14 24F-V4 DOUGLAS FIR GLU-LAM BEAM	5 7/16" X 14"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BGLU6*	5 7/16X16 24F-V4 DOUGLAS FIR GLU-LAM BEAM	5 7/16" X 16"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BGLU7*	5 7/16X18 24F-V4 DOUGLAS FIR GLU-LAM BEAM	5 7/16" X 18"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BGLU8*	7 X14 24F-V4 DOUGLAS FIR GLU-LAM BEAM	7" X 14"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BGLU9*	7 X16 24F-V4 DOUGLAS FIR GLU-LAM BEAM	7" X 16"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BGLU10*	7 X18 24F-V4 DOUGLAS FIR GLU-LAM BEAM	7" X 18"	DOUGLAS FIR	SEE A-161 / A-272 / A-273
BPSL1*	3 1/2 X 9 1/4 2.OE PARALLAM PSL	3 1/2" X 9 1/4"	2.OE	SEE A-161 / A-272 / A-273
BPSL2*	3 1/2X11 1/4 2.OE PARALLAM PSL	3 1/2" X 11 1/4"	2.OE	SEE A-161 / A-272 / A-273
BPSL3*	5 1/4X9 1/4 2.OE PARALLAM PSL	5 1/4" X 9 1/4"	2.OE	SEE A-161 / A-272 / A-273
BPSL4*	5 1/4X11 1/4 2.OE PARALLAM PSL	5 1/4" X 11 1/4"	2.OE	SEE A-161 / A-272 / A-273
BPSL5*	5 1/4X14 2.OE PARALLAM PSL	5 1/4" X 14"	2.OE	SEE A-161 / A-272 / A-273
BPLS6*	5 1/4X16 2.OE PARALLAM PSL	5 1/4" X 16"	2.OE	SEE A-161 / A-272 / A-273
BPSL7*	7X9 1/4 2.OE PARALLAM PSL	7" X 9 1/4"	2.OE	SEE A-161 / A-272 / A-273
BPSL8*	7X11 1/4 2.OE PARALLAM PSL	7" X 11 1/4"	2.OE	SEE A-161 / A-272 / A-273
BPSL9*	7X14 2.OE PARALLAM PSL	7" X 14"	2.OE	SEE A-161 / A-272 / A-273
BPSL10*	7X16 2.OE PARALLAM PSL	7" X 16"	2.OE	SEE A-161 / A-272 / A-273

Typical Wood Beam Schedule (2 of 2)

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ID	DESCRIPTION	ACTUAL DIMENSION	GRADE/ SPECIES	CONNECTORS
BLVL1*	1 3/4X5 1/2 1.9E WS MICROLLAM LVL	1 3/4" X 5 1/2"	1.9E	SEE MANUFACTURE'S SPECS.
BLVL2*	1 3/4X7 1/4 1.9E WS MICROLLAM LVL	1 3/4" X 7 1/4"	1.9E	SEE MANUFACTURE'S SPECS.
BLVL3*	1 3/4X9 1/4 1.9E WS MICROLLAM LVL	1 3/4" X 9 1/4"	1.9E	SEE MANUFACTURE'S SPECS.
BLVL4*	1 3/4X11 1/4 1.9E WS MICROLLAM LVL	1 3/4" X 11 1/4"	1.9E	SEE MANUFACTURE'S SPECS.
BLVL5*	1 3/4X14 1.9E WS MICROLLAM LVL	1 3/4" X 14"	1.9E	SEE MANUFACTURE'S SPECS.
BLVL6*	1 3/4X14 1.9E WS MICROLLAM LVL	1 3/4" X 16"	1.9E	SEE MANUFACTURE'S SPECS.

* ALL BEAMS TO BE VERIFIED BY MANUFACTURE/SUPPLIER ENGINEER BEFORE INSTALLATION.
ANY RESIZING OF BEAMS MUST BE APPROVED BY CONTRACTOR/ENGINEER AND LOCAL
BUILDING DEPARTMENT PRIOR TO INSTALLATION.

Typical Wood Post Schedule

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ID	DESCRIPTION	ACTUAL DIMENSION	GRADE/ SPECIES	CONNECTORS
P4X4	4X4 DOUGLAS FIR #2 POST	3 1/2" X 3 1/2"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
P4X6	4X6 DOUGLAS FIR #2 POST	3 1/2" X 5 1/2"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
P4X8	4X8 DOUGLAS FIR #2 POST	3 1/2" X 7 1/2"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
P6X6	6X6 DOUGLAS FIR #2 POST	5 1/2" X 5 1/2"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
P6X8	6X8 DOUGLAS FIR #2 POST	5 1/2" X 7 1/2"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
B8X8	8X8 DOUGLAS FIR #2 POST	7 1/2" X 7 1/2"	#2 OR BETTER D.F.	SEE A-161 / A-272 / A-273
PT4X4	PRESSURE TREATED 4X4 DOUGLAS FIR #2 POST	3 1/2" X 3 1/2"	PRESSURE TREATED	SEE A-161 / A-272 / A-273
PT4X6	PRESSURE TREATED 4X6 DOUGLAS FIR #2 POST	3 1/2" X 5 1/2"	PRESSURE TREATED	SEE A-161 / A-272 / A-273
PT4X8	PRESSURE TREATED 4X8 DOUGLAS FIR #2 POST	3 1/2" X 7 1/2"	PRESSURE TREATED	SEE A-161 / A-272 / A-273
PT6X6	PRESSURE TREATED 6X6 DOUGLAS FIR #2 POST	5 1/2" X 5 1/2"	PRESSURE TREATED	SEE A-161 / A-272 / A-273
PT6X8	PRESSURE TREATED 6X8 DOUGLAS FIR #2 POST	5 1/2" X 7 1/2"	PRESSURE TREATED	SEE A-161 / A-272 / A-273
BT8X8	PRESSURE TREATED 8X8 DOUGLAS FIR #2 POST	7 1/2" X 7 1/2"	PRESSURE TREATED	SEE A-161 / A-272 / A-273

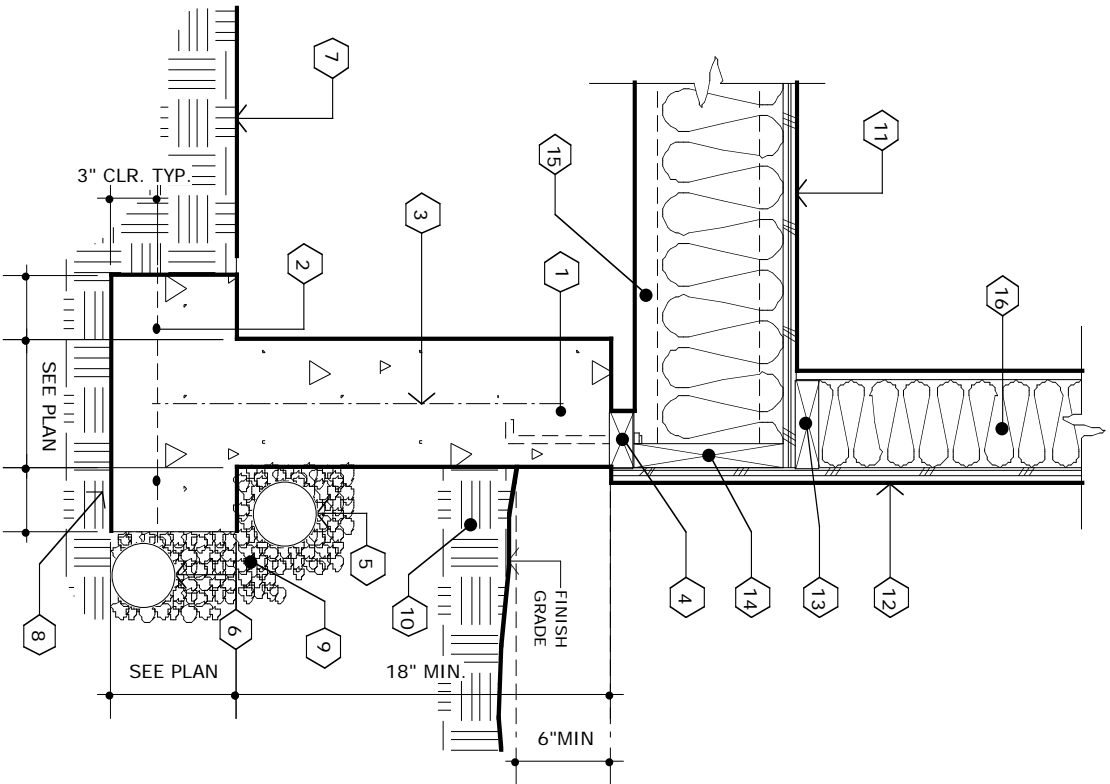
Wood Shearwall Schedule

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Mark	Sheathing	Connection to wood	Connection to Concrete
SW-1	7/16" O.S.B. w/ M.N.P. W/ 8d @ 6" O.C. EN, @ 12" O.C. FN	16d @ 5"O.C.	1/2" DIA A.B. @ 32" O.C.
SW-2	7/16" O.S.B. w/ M.N.P. W/ 8d @ 4" O.C. EN, @ 12" O.C. FN	16d @ 4"O.C.	1/2" DIA A.B. @ 24" O.C.
SW-3	7/16" O.S.B. Plywood w/ M.N.P. W/ 8d @ 3" O.C. EN, @ 12" O.C. FN	16d @ 3"O.C.	1/2" DIA A.B. @ 16" O.C.
SW-4	1/2" CDX Plywood w/ M.N.P. = 1 5/8" W/ 10d @ 3" O.C. EN, @ 12" O.C. FN	16d @ 2 1/2"O.C. (or 2 rows @ 5" O.C.)	3/4" DIA A.B. @ 12" O.C.
SW-5	1/2" CDX Plywood w/ M.N.P. = 1 5/8" W/ 10d @ 2" O.C. EN, @ 8" O.C. FN	16d @ 5"O.C. (or 2 rows @ 4" O.C.)	3/4" DIA A.B. @ 24" O.C.
SW-6	1/2" CDX Plywood w/ M.N.P. = 1 5/8" W/ 10d @ 2" O.C. EN, @ 6" O.C. FN	2-16d @ 3" O.C.	1/2" DIA A.B. @ 16" O.C.

Notes:

- (1) 15/32" Plywood may be considered as 1/2" plywood
- (2) P.E.N. = Plywood edge nailers (Studs &/or joist &/or blocking)
- (3) M.N.P. = Min nail penetration on framing = 1/2" U.N.O.
- (4) Sill connection nailing to wood may be reduced to 16d @ 4" O.C. Where plywood sheathing E.N. Extends past sill plates to ledger or rim joist



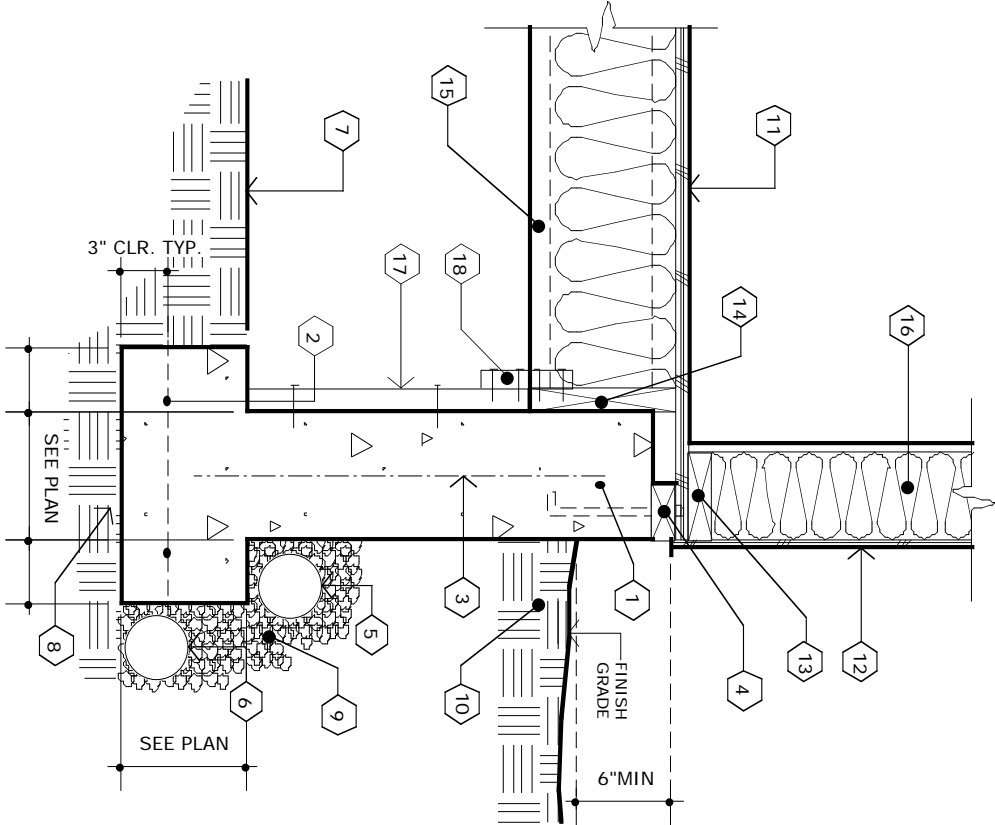
NOTES

- < 1 > (1) #4 CONT. TOP BAR, U.N.O.
- < 2 > (2) #4 CONT. BOTTOM BARS U.N.O.
- < 3 > #4 VERTICAL REBAR @18" O.C., U.N.O.
- < 4 > 2X PRESSURE TREATED SILL (SEE SHEAR WALL SCHEDULE FOR ANCHOR BOLT SIZE AND SPACING.) ALSO SEE <3> A-102.
- < 5 > TIGHTLINE STORM DRAIN.
- < 6 > 4" DIA. PERFORATED FOUNDATION DRAIN
- < 7 > 6 MIL. VB. MIN.
- < 8 > UNDISTURBED NATURAL SOIL OR ENGINEERED FILL.
- < 9 > DRAINAGE GRADE GRAVEL.
- < 10 > RELATIVELY IMPERMEABLE LAYER TOP SOIL.
- < 11 > 3/4" T&G PLYWOOD SUBFLOOR.
- < 12 > SIDING PER ELEVATIONS, #15 BUILDING PAPER, 1/2" CDX PLYWOOD SHEATHING.
- < 13 > 2X6 SILL PLATE.
- < 14 > RIM JOIST PER PLAN.
- < 15 > JOIST PER PLAN WITH BATT INSULATION.
- < 16 > 2X6 STUDS @ 16" O.C. W/ BATT INSULATION.

REV	DATE	DESCRIPTION
PROJECT		
DRAWING DESCRIPTION		
TYPICAL THRU-FOUNDATION, THRU-JOIST, THRU-WALL SECTION		
DRAWING NO.		
SCALE		
1" = 1'-0"		
CREATION DATE		
5/1/11		
DRAWN BY		
John-Paul R. Cox, Assoc. AIA		
Do not scale drawings. All dimensions should be checked on site. The ownership of the copyright in this drawing is retained by the Architects whose consent must be obtained before any use or reproduction of the drawing or any part thereof can be made.		

NOTES

- <1> (1) #4 CONT. TOP BAR, U.N.O.
- <2> (2) #4 CONT. BOTTOM BARS U.N.O.
- <3> #4 VERTICAL REBAR @18" O.C., U.N.O.
- <4> 2X PRESSURE TREATED SILL (SEE SHEAR WALL SCHEDULE FOR ANCHOR BOLT SIZE AND SPACING.) ALSO SEE <3> A-102.
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- <7> 6 MIL. VB. MIN.
- <8> UNDISTURBED NATURAL SOIL OR ENGINEERED FILL.
- <9> DRAINAGE GRADE GRAVEL.
- <10> RELATIVELY IMPERMEABLE LAYER TOP SOIL.
- <11> 3/4" T&G PLYWOOD SUBFLOOR.
- <12> SIDING PER ELEVATIONS. #15 BUILDING PAPER. 1/2" CDX PLYWOOD SHEATING.
- <13> 2X6 SILL PLATE.
- <14> RIM JOIST PER PLAN.
- <15> JOIST PER PLAN WITH BATT INSULATION.
- <16> 2X6 STUDS @ 16" O.C. W/ BATT INSULATION.
- <17> 2X4 PRESSURE TREATED 24" O.C. DURA-NAILED 24" OC. TO FOUNDATION
- <18> 1X4 CLEAT NAILED WITH 16D @ 4" O.C.



REV DATE DESCRIPTION

PROJECT

DRAWING DESCRIPTION
TYPICAL "SUNKEN JOISTS" THRU-FOUNDATION, THRU-JOIST, THRU-WALL SECTION, THRU-SLAB.

DRAWING NO.

A-102

SCALE

1" = 1'-0"

CREATION DATE

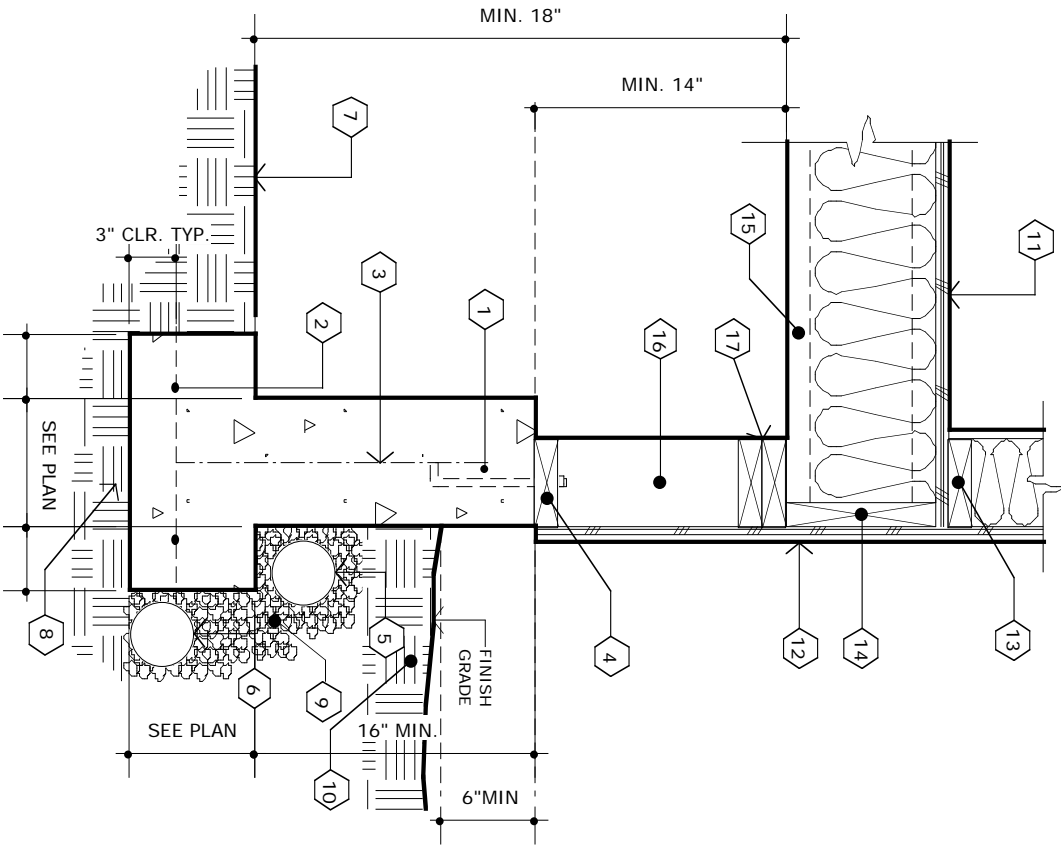
5/11/11

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John-Paul R. Cox, Assoc. AIA

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NOTES

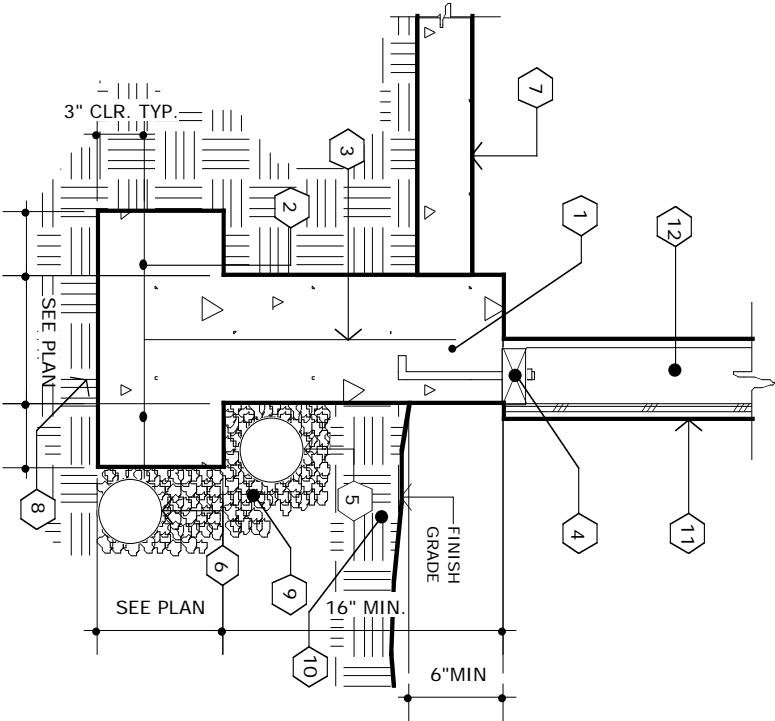
- < 1 > (1) #4 CONT. TOP BAR, U.N.O.
- < 2 > (2) #4 CONT. BOTTOM BARS U.N.O.
- < 3 > #4 VERTICAL REBAR @18" O.C., U.N.O.
- < 4 > 2X PRESSURE TREATED SILL (SEE SHEAR WALL SCHEDULE FOR ANCHOR BOLT SIZE AND SPACING.) ALSO SEE < 3 > A-102.
- < 5 > TIGHTLINE STORM DRAIN.
- < 6 > 4" DIA. PERFORATED FOUNDATION DRAIN
- < 7 > 6 MIL. VB. MIN.
- < 8 > UNDISTURBED NATURAL SOIL OR ENGINEERED FILL.
- < 9 > DRAINAGE GRADE GRAVEL.
- < 10 > RELATIVELY IMPERMEABLE LAYER TOP SOIL.
- < 11 > 3/4" T&G PLYWOOD SUBFLOOR.
- < 12 > SIDING PER ELEVATIONS. #15 BUILDING PAPER. 1/2" CDX PLYWOOD SHEATING.
- < 13 > 2X6 SILL PLATE.
- < 14 > RIM JOIST PER PLAN.
- < 15 > JOIST PER PLAN WITH BATT INSULATION.
- < 16 > 2X6 STUDS @ 16" O.C.
- < 17 > (2) 2X6 TOP PLATES

REV	DATE	DESCRIPTION
PROJECT		
DRAWING DESCRIPTION		
TYPICAL "PONY WALL" THRU- FOUNDATION, THRU-JOIST, THRU- WALL SECTION.		

DRAWING NO.	
A-104	
SCALE	
1" = 1'-0"	
CREATION DATE	
5/1/11	
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NOTES

- < 1> (1) #4 CONT. TOP BAR, U.N.O.
- < 2> (2) #4 CONT. BOTTOM BARS U.N.O.
- < 3> #4 VERTICAL REBAR @18" O.C., U.N.O.
- < 4> 2X PRESSURE TREATED SILL (SEE SHEAR WALL SCHEDULE FOR ANCHOR BOLT SIZE AND SPACING.) ALSO SEE <3> A-102.
- < 5> TIGHTLINE STORM DRAIN.
- < 6> 4" DIA. PERFERATED FOUNDATION DRAIN
- < 7> 4" CONCRETE SLAB W/ 6X6 #10 WWM O/4 MIL. V.B. (OPTIONAL GRANULAR FILL UNDER SLAB)
- < 8> UNDISTURBED NATURAL SOIL OR ENGINEERED FILL.
- < 9> DRAINAGE GRADE GRAVEL.
- < 10> RELATIVELY IMPERMEABLE LAYER TOP SOIL.
- < 11> SIDING PER ELEVATIONS. #15 BUILDING PAPER. 1/2" CDX PLYWOOD SHEATHING.
- < 12> 2X STUDS SPACED 16" O.C.



REV DATE DESCRIPTION

PROJECT

DRAWING DESCRIPTION

TYPICAL GARAGE FOUNDATION SECTION

DRAWING NO.

A-105

SCALE

1" = 1'-0"

CREATION DATE

5/1/11

DRAWN BY

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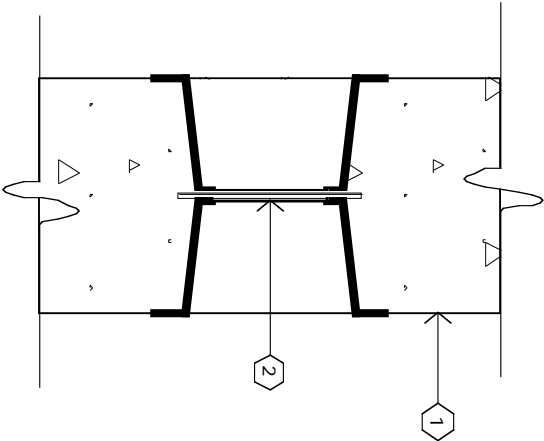
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REV	DATE	DESCRIPTION

PROJECT

NOTES

- < 1 > 8" CONCRETE FOUNDATION WALL.
- < 2 > 8X8X16 FOUODANTION VENT WITH SCREEN.



DRAWING DESCRIPTION
TYPICAL FOUNDATION VENT SECTION

DRAWING NO.

A-106

SCALE

1" = 1'-0"

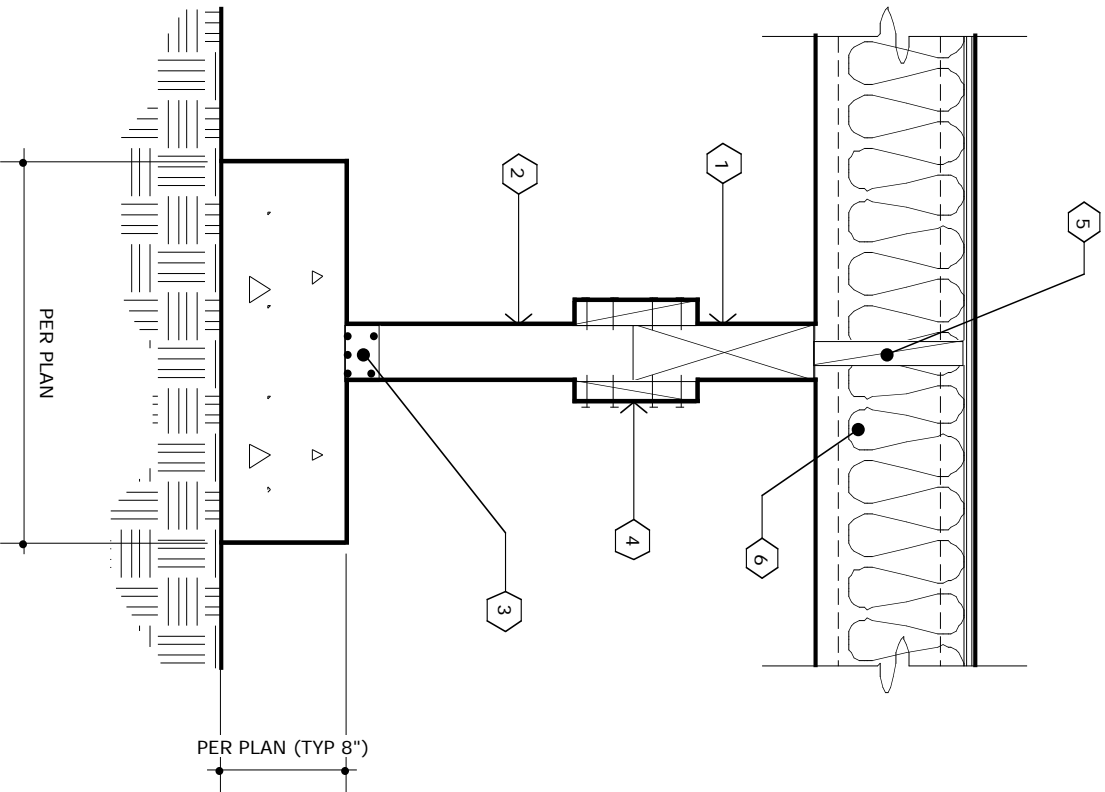
CREATION DATE

5/1/11

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NOTES

- < 1> BEAM PER PLAN.
(4X12 TYP, U.N.O.)
- < 2> POST PER PLAN
(4X4 TYP, U.N.O.)
- < 3> SIMPSON BC MATCHING POST
BASE. (BC___) INSTALL PER
MFGR. SPECS. (SEE A-161)
- < 4> 2X4 CLEAT E.F., NAIL TO
<1> AND <2> MINIMUM
4 - 16D NAILS E.F., TOP AND
BOTTOM.
- < 5> SOLID BLOCKING OVER
ALL BEAMS
- < 6> FLOOR JOIST PER PLAN
W/ BATT INSULATION.

REV	DATE	DESCRIPTION
PROJECT		

DRAWING DESCRIPTION
TYPICAL THRU-PAD, THRU-POST,
THRU-BEAM, THRU JOIST SECTION.

DRAWING NO.

A-120

SCALE

1" = 1'-0"

CREATION DATE

5/1/11

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STHD STRAP TIE
HOLDOWN

The STHD is an embedded strap tie holdown with higher load capacity than the HPARD. The staggered nail pattern helps to minimize splitting.

FEATURES: • The strap nailing pattern allows for nailing to the edges of double 2x's. • A slot below the embedment line allows for increased front to back concrete bond and reduced spalling.

- Strap nail slots are countersunk to provide a lower nail head profile.
- Rim joist models accommodate up to a 17" clear span without any loss of strap nailing.

• Corneled edges enhance safe handling.

MATERIAL: LSTDH8 / LSTDH8RJ—14 gauge, all others—12 gauge.

FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

- See Post Tension Information on page 19.

• Nail strap from the bottom up.

• Bending the strap 90° to aid wall placement may cause spalling behind the strap. If the spill is 1" or less, measured from the top of the concrete foundation to the bottom of the spill, full loads apply. For slabs between 1" and 4" (see illustration), the allowable load is 0.90 of the table loads.

- Where lever fasteners are used in the structural wood member, reduce loads according to the code.

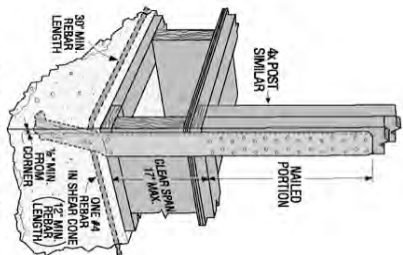
• Unless otherwise noted, do NOT install where:

- (a) a horizontal void joint exists within the embedment depth between the slab and foundation wall or footing beneath, unless provisions are made to transfer the load, or the slab is designed to resist the load imposed by the anchor, or
- (b) slabs are poured over concrete block foundation walls.

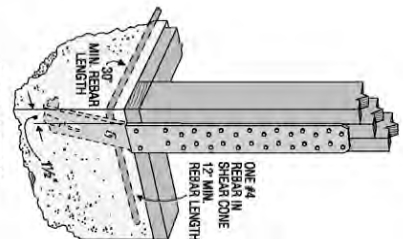
- To get the full table load, the min. center-to-center spacing is twice the embedment depth when resisting tension loads at the same time.

FOUNDATION CORNERS: Nail quantities may be reduced for less than 16" corner distance design loads—use the code allowable loads for fasteners in shear.

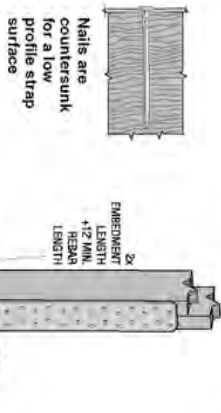
CODE: IC80 5349



Typical STHD14RJ
Rim Joist Application



Typical STHD
Corner Installation
on 3-2x studs
(for 2 pour, see
footnote 4.)



Typical
STHD Edge
Installation
(for 2 pour,
see footnote 4.)

Model No. Standard / Rim Joist	Min Stem Wall	Strap Length (l) Sld Model	l _e	Nails	Avg UH @ 2000 psi l _e	Allowable loads (133 & 160)											
						Edge Distance											
						1/2"	1 1/8"	l _e 2	1/2"	1 1/8"	l _e 2						
2000 psi Concrete												2500 psi Concrete			3000 psi Concrete		
LSTDH8 / LSTDH8RJ	6	21%	35%	8	24-16d sinker	5918	1695	1695	1695	1825	1825	1825	1950	1950	1950		
STHD8 / STHD8RJ	6	21%	35%	8	24-16d sinker	7167	1760	2050	2345	1950	2210	2385	2135	2370	2425		
STHD10 / STHD10RJ	6	23%	36%	10	28-16d sinker	10555	2035	2575	3185	2610	2880	3185	3185	3185	3185		
STHD14 / STHD14RJ	6	31%	39%	14	38-16d sinker	15080	3235	4220	4805	3800	4295	4805	4365	4365	4805		
LSTDH8 / LSTDH8RJ	8	21%	35%	8	24-16d sinker	5918	1695	1695	1695	1825	1825	2335	1950	1950	2975		
STHD8 / STHD8RJ	8	21%	35%	8	24-16d sinker	7577	2370	2370	3195	2370	2370	3195	2370	2370	3195		
STHD10 / STHD10RJ	8	23%	36%	10	28-16d sinker	11783	2745	2745	3725	2990	2990	3725	3230	3230	3725		
STHD14 / STHD14RJ ¹	8	31%	39%	14	38-16d sinker	17453	3885	4430	5785	4160	4430	5785	4430	4430	5785		

1. 'RJ' after the model indicates STHDs for rim joist applications, e.g. STHD8RJ.
2. STHD14RJ requires 30-16d sinkers with the (le) load at 133% of 4960 lbs.

3. 10d commons or 12d common nails may be used with no load reduction.
4. For two pour with 4" slab or less, install STHD14 and use STHD10 loads.

REV DATE DESCRIPTION

PROJECT		

DRAWING DESCRIPTION

SIMPSON STHD 14 (RJ) HOLD-DOWN DIAGRAM.

DRAWING NO.

A-151

SCALE

NOT TO SCALE

CREATION DATE

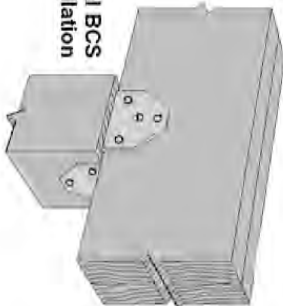
5/1/11

DRAWN BY

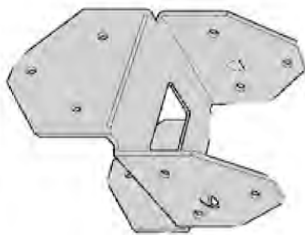
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Typical BCS Installation



BCS2-2/4

U.S. Patent Nos.
4,480,941 and 5,603,580
Canada Patent 1,193,418

BC POST CAPS

The BCS allows for the connection of 2-2x's to a 4x post or 3-2x's to a 6x post. Double shear nailing between beam and post gives added strength!

The BC series offers dual purpose post cap/base for light cap or base connections.

MATERIAL: 18 gauge

FINISH: Galvanized

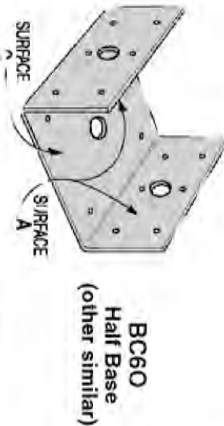
INSTALLATION: • Use all specified fasteners. See General Notes.

• BCS: install dome nails on beam, drive nails at an angle through the beam into the post below to achieve the table loads

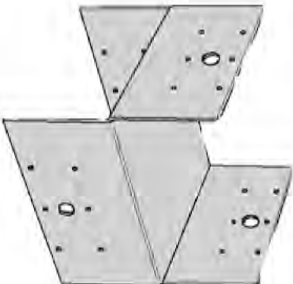
• BC: install with 16d commons or 16dx2" joist hanger nails.

• Not recommended for non-top-supported installations such as fences when used as a base.

CODES: BOCA, ICBO, SBCOI NER-421, ICBO 5357, Dade County FL 97-0325.03, City of L.A. R.R 25076 (except BCO).
BOCA 97-9 (BCS).



BC60
Half Base
(other similar)



BC8 Cap/Base

Model No.	Dimensions						Fasteners (Each Side)			Uplift Avg Upl	Allowable Loads (133 & 160) ¹	
	W ₁	W ₂	L ₁	L ₂	H ₁	H ₂	Surface A	Surface B	Surface C		Uplift	Lateral
CAPS												
BC4	3 $\frac{1}{8}$	3 $\frac{1}{8}$	2 $\frac{1}{2}$	2 $\frac{1}{2}$	3	3	3-16d	3-16d	—	3100	980	1000
BC46	3 $\frac{1}{8}$	5 $\frac{1}{2}$	4 $\frac{1}{2}$	2 $\frac{1}{2}$	3 $\frac{1}{2}$	2 $\frac{1}{2}$	6-16d	3-16d	—	3100	980	1000
BC4R	4	4	4	4	3	3	6-16d	6-16d	—	3100	980	1000
BC6	5 $\frac{1}{2}$	5 $\frac{1}{2}$	4 $\frac{1}{2}$	4 $\frac{1}{2}$	3 $\frac{1}{2}$	3 $\frac{1}{2}$	6-16d	6-16d	—	4700	1050	2000
BC6R	6	6	6	6	3	3	6-16d	6-16d	—	4700	1050	2000
BC8	7 $\frac{1}{2}$	7 $\frac{1}{2}$	7 $\frac{1}{2}$	7 $\frac{1}{2}$	4	4	6-16d	6-16d	—	5600	1800	2000
BCS2-2/4	3 $\frac{1}{8}$	3 $\frac{1}{8}$	2 $\frac{1}{2}$	2 $\frac{1}{2}$	2 $\frac{1}{2}$	2 $\frac{1}{2}$	4-10d	3-10d	—	2697	780	1025
BCS2-3/6	4 $\frac{1}{8}$	5 $\frac{1}{8}$	4 $\frac{1}{8}$	2 $\frac{1}{2}$	3 $\frac{1}{8}$	2 $\frac{1}{8}$	6-16d	3-16d	—	3000	800	1495
BASES												
BC40	3 $\frac{1}{8}$	—	3 $\frac{1}{2}$	—	2 $\frac{1}{2}$	—	3-16d	—	4-16d	—	—	535
BC40R	4	—	4	—	3	—	4-16d	—	4-16d	—	—	535
BC460	5 $\frac{1}{2}$	—	3 $\frac{1}{2}$	—	3	—	4-16d	—	4-16d	—	—	535
BC60	5 $\frac{1}{2}$	—	5 $\frac{1}{2}$	—	3	—	6-16d	—	4-16d	—	—	535
BC60R	6	—	6	—	3	—	6-16d	—	4-16d	—	—	535

¹ Allowable loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed, reduce for other load durations according to the code

REV DATE DESCRIPTION

PROJECT

DRAWING DESCRIPTION

SIMPSON BC POST CAPS.

DRAWING NO.

A-161

SCALE

NOT TO SCALE

CREATION DATE

5/1/11

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NOTES

- < 1> (2) 2X6 STUDS
- < 2> RIM JOIST PER PLAN
- < 3> 1/2" CDX PLYWOOD SHEATING,
#15 BUILDING PAPER.
- < 4> (2) 2X6 TOP PLATE
- < 5> 2X6 PRESURE TREATED
SILL PLATE (SEE SHEAR WALL
SCHEDULE FOR ANCHOR
BOLT SIZE AND SPACING.
- < 6> 2X6 STUDS SPACED 16" O.C.

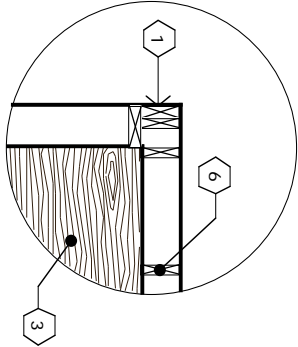
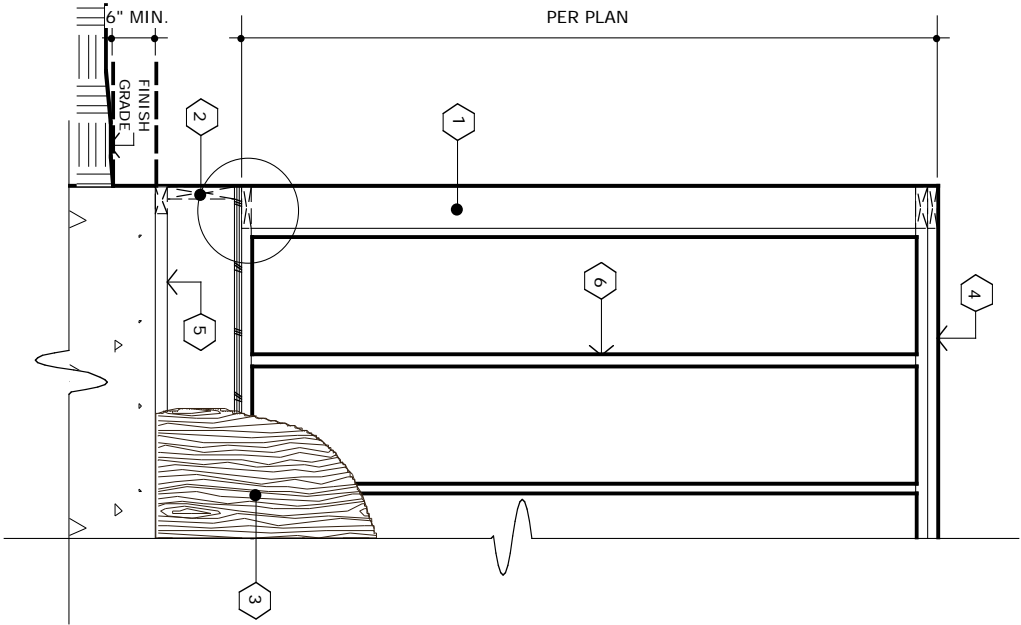


DIAGRAM - A 3/4:1



REV	DATE	DESCRIPTION

PROJECT

DRAWING DESCRIPTION
TYPICAL 2X6 FRAMED WALL
DIAGRAM

DRAWING NO.

A-201

SCALE

NOT TO SCALE

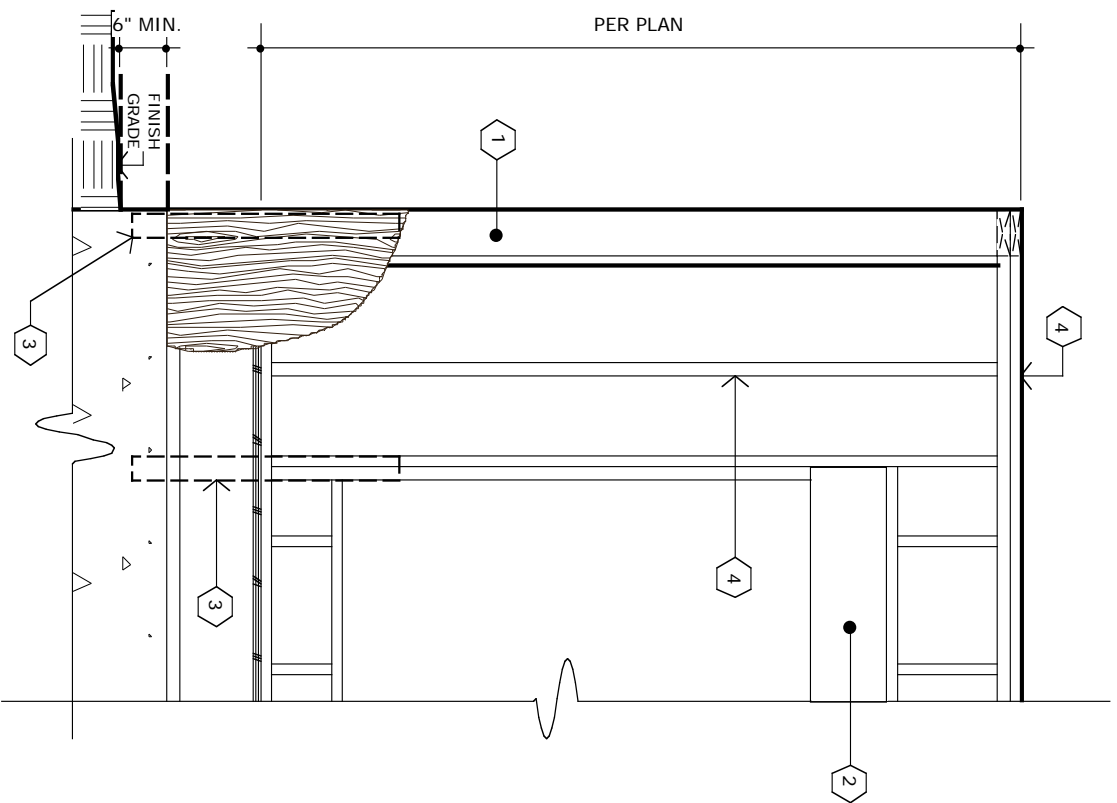
CREATION DATE

5/1/11

DRAWN BY

John-Paul R. Cox, Assoc. AIA

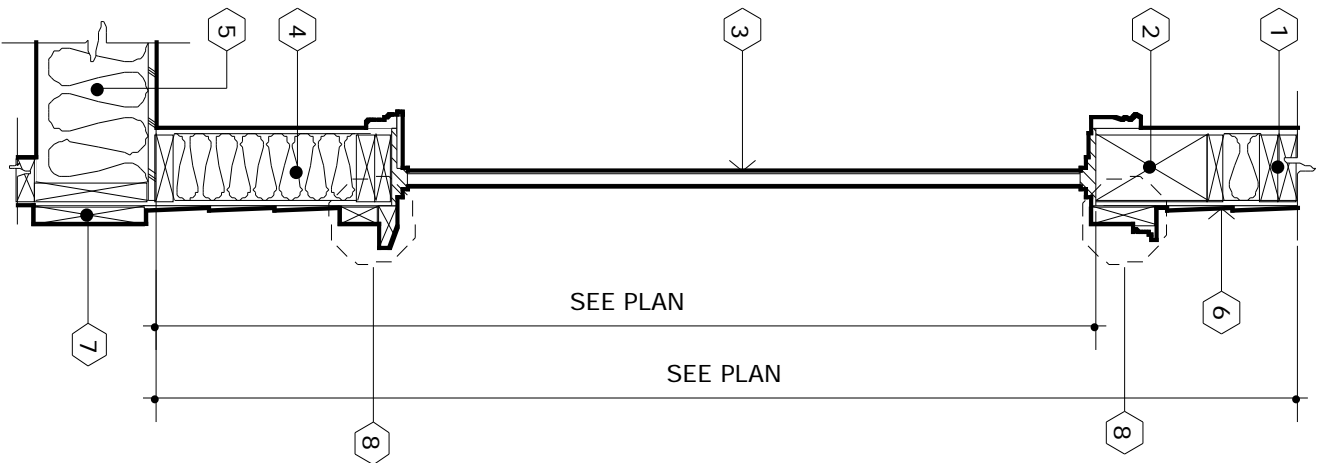
Do not scale drawings. All dimensions should be checked on site. The ownership of the copyright in this drawing is retained by the Architects whose consent must be obtained before any use or reproduction of the drawing or any part thereof can be made.



NOTES

- < 1> (2) 2X6 STUDS
- < 2> 6X10 D.F. # 2 OR BETTER HEADER (UNLESS SPECIFIED DIFFERENT)
- < 3> SIMPSON HOLD-DOWNS FOR ALTERNATIVE BRACE WALL PANEL. (SEE MFG. SPECS. FOR INSTALLATION)
- < 4> 2X6 STUDS SPACED 16" O.C.

REV	DATE	DESCRIPTION
PROJECT		
DRAWING DESCRIPTION		
TYPICAL WALL WITH HEADER AND HOLD-DOWNS DIAGRAM.		
DRAWING NO.		
A-202		
SCALE		
NOT TO SCALE		
CREATION DATE		
5/1/11		
DRAWN BY		
John-Paul R. Cox, Assoc. AIA		
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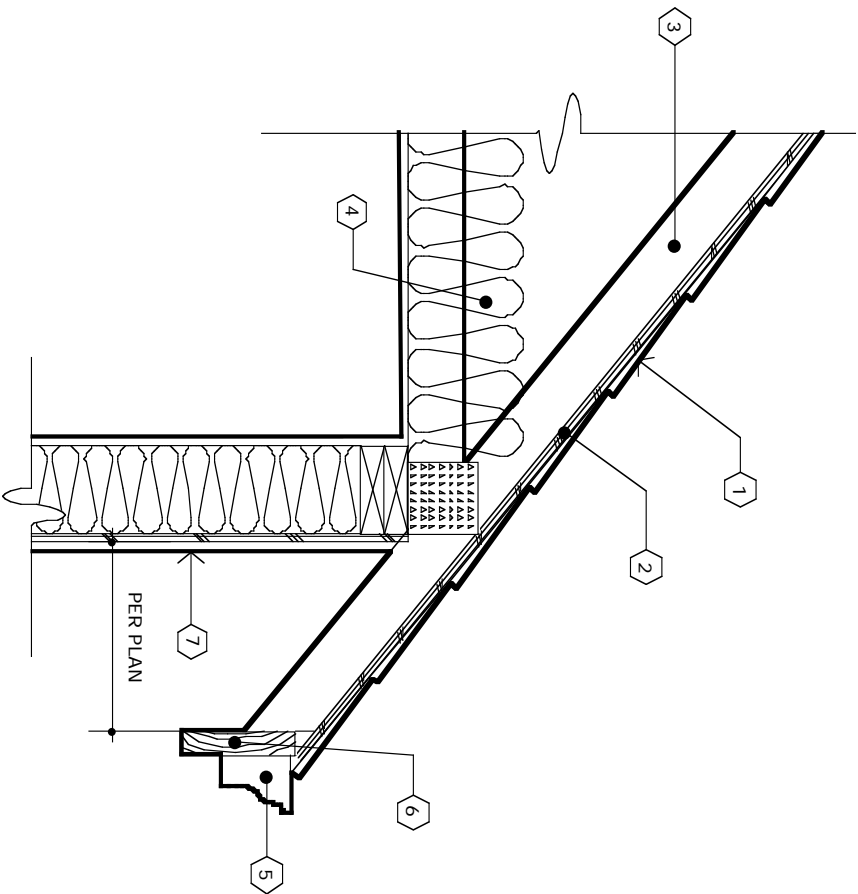
NOTES

- < 1> (2) 2X TOP PLATE.
- < 2> 6X HEADER. (6X10 D.F TYP)
- < 3> WINDOW PER PLAN.
- < 4> 2X6 STUDS @ 16" O.C. WITH BATT INSULATION.
- < 5> FLOOR JOIST PER PLAN WITH BATT INSULATION.
- < 6> SIDING PER ELEVATIONS. #15 BUILDING PAPER. 1/2" CDX PLYWOOD SHEATHING.
- < 7> OPTIONAL 2X10 BEALLY-BAN.
- < 8> EXTERIOR TRIM PER PLAN.

REV	DATE	DESCRIPTION
PROJECT		
DRAWING DESCRIPTION		
TYPICAL EXTERIOR WALL SECTION.		
DRAWING NO.		
A-221		
SCALE		
3/4" = 1'-0"		
CREATION DATE		
5/1/11		
DRAWN BY		
John-Paul R. Cox, Assoc. AIA		
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NOTES

- < 1> ROOF MATERIAL PER PLAN.
- < 2> **IF COMPOSITION:**
30# FELT INTERWOVEN
1/2" PLYWOOD SHEATHING
IF CEDAR SHAKE:
30# FLET INTERWOVEN
1X4 SLATS SPACED 12" O.C.
- < 3> TRUSS PER MFR. PLAN
- < 4> BLOWN IN INSULATION.
- < 5> 5" CONTINUOUS MTL. GUTTER.
- < 6> 2X8 CEDAR FACIA.
- < 7> SIDING PER ELEVATION.



REV	DATE	DESCRIPTION
PROJECT		

DRAWING DESCRIPTION
TYPICAL MFR. TRUSS ROOF EVE
FRAMING SECTION.

DRAWING NO.

A-222

SCALE

1" = 1'-0"

CREATION DATE

5/1/11

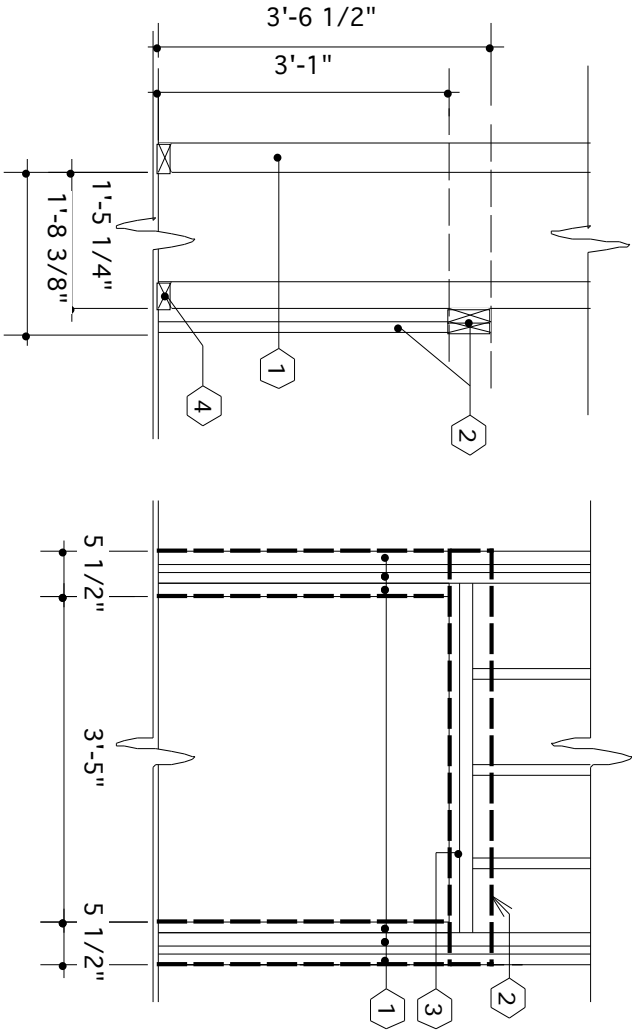
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NOTES

- < 1 > 2X4 INTERIOR STUDS.
- < 2 > (2) 2X6 RIM.
- < 3 > (2) 2X4 OPENING HEADER.
- < 4 > 2X4 PLATE.



REV	DATE	DESCRIPTION

PROJECT

DRAWING DESCRIPTION
SINGLE SIDE GAS APPLANCE
FIREPLACE ROUGH FRAMING.

DRAWING NO.

A-242

SCALE

1/2" = 1'-0"

CREATION DATE

5/1/11

DRAWN BY

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POST
CAPS

A custom connection for post-beam combinations at medium design loads.

MATERIAL: PC—12 gauge; PC-16—16 gauge

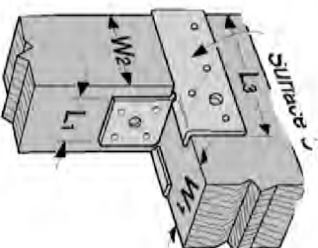
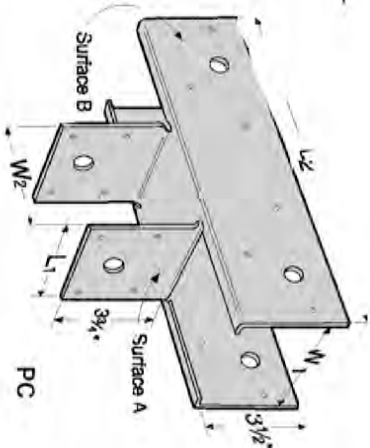
FINISH: Galvanized. Some products available in Z-MAX; see Corrosion-Resistance, page 6.

INSTALLATION: • Use all specified fasteners; see General Notes
• D/Cables are provided for optional bolting. Loads do not apply to bolted connection.

OPTIONS: • For end conditions, specify EPC post caps, providing dimensions are in accordance with table; see illustration.

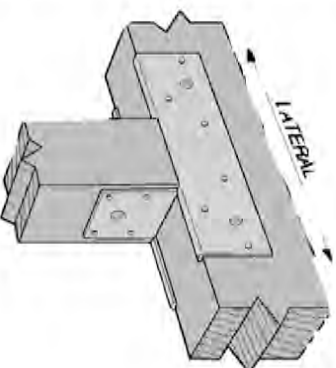
- PC and EPC models are available in rough sizes
- For heavy duty applications, see also CC series.

CODES: BOCA, ICBO SBCCI NER-443; City of L.A. RR 25149.



Typical EPC End Post Cap Installation

Model No.	Post Size	DIMENSIONS					Fasteners each side			Allowable Loads					
		W ₁	W ₂	L ₁	L ₂	L ₃	Surfaces			Uplift Avg U _{int}	Lateral :				
							A	B	C		Uplift PC/EPC (133) (160) (133/160)	PC (133/160)	EPC (133/160)		
PC44-16	4X4	3%	3%	2%	11	7%	4-16d	6-16d	4-16d	3433	1000	1000	925	1000	
PC44	4X4	3%	3%	2%	11	7%	4-16d	6-16d	4-16d	4933	1470	1700	925	1070	
PC46-16	4X6	3%	5%	2%	13	9%	4-16d	6-16d	4-16d	3433	1000	1000	925	1000	
PC46	4X6	3%	5%	2%	13	9%	4-16d	6-16d	4-16d	4933	1470	1700	925	1070	
PC48-16	4X8	3%	7%	2%	15	11%	4-16d	8-16d	6-16d	3433	1000	1000	1475	1285	
PC48	4X8	3%	7%	2%	15	11%	4-16d	8-16d	6-16d	4933	1470	1700	2075	1610	
PC64-16	4X6	5%	3%	4%	11	7%	4-16d	6-16d	4-16d	3433	1000	1000	925	1000	
PC64	4X6	5%	3%	4%	11	7%	4-16d	6-16d	4-16d	4933	1470	1700	925	1070	
PC66-16	6X6	5%	5%	4%	13	9%	4-16d	6-16d	6-16d	3433	1000	1000	925	1285	
PC66	6X6	5%	5%	4%	13	9%	4-16d	6-16d	6-16d	4933	1470	1700	925	1610	
PC68	6X8	5%	7%	4%	15	11%	4-16d	8-16d	6-16d	4933	1470	1700	2075	1610	
PC84	4X8	7%	3%	6%	11	7%	4-16d	6-16d	6-16d	4933	1470	1700	925	1610	
PC86	6X8	7%	5%	6%	13	9%	4-16d	6-16d	6-16d	4933	1470	1700	925	1610	
CC88	8X8	7%	7%	6%	15	11%	4-16d	8-16d	6-16d	4933	1470	1700	2075	1610	



Typical PC Post Cap Installation

1. Allowable loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed; reduce for other load durations according to the code
2. Lateral loads are in the direction of the beam's axis, as shown
3. Allowable loads are for nails only.

REV	DATE	DESCRIPTION
PROJECT		

DRAWING DESCRIPTION

SIMPSON PC POST CAPS.

DRAWING NO

A-272

SCALE

NTS

CREATION DATE

5/1/11

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OBRICK | COX

AC/LPC POST CAPS

The AC MAX design allows for higher load capacity to match comparable post bases.

AC—Twin design allows easy installation.

LPC—Adjustable design allows greater connection versatility.

MATERIAL: AC, LPC4—18 gauge; LPC6—16 gauge

FINISH: Galvanized. Some products available with Z-MAX; see Corrosion-Resistance, page 6.

INSTALLATION • Use all specified fasteners. See General Notes.

• Install AC and LPC in pairs. LPC—2x4 beams may be used if

10dx1x10 nails are substituted for 10d commons.

CODES: BOCA, ICB0, SBCCI NER-421, NER-443, NER-469, Dade

County, FL 97-0325.03; City of L.A., RR 25076.

Model No.	Dimensions		Fasteners		Uplift Avg. Unit	Allowable Loads (133 & 160) ¹	
	W	L	Beam	Post		Uplift	Lateral
AC4 MIN	3 $\frac{3}{8}$	6 $\frac{1}{2}$	12-16d	8-16d	4467	1430	715
AC4 MAX	3 $\frac{3}{8}$	6 $\frac{1}{2}$	14-16d	14-16d	10000	2500	1070
AC4R MIN	4	7	12-16d	8-16d	4467	1430	715
AC4R MAX	4	7	14-16d	14-16d	10000	2500	1070
ACE4 MIN	—	4 $\frac{1}{2}$	8-16d	6-16d	—	1070	715
ACE4 MAX	—	4 $\frac{1}{2}$	10-16d	10-16d	—	1785	1070
AC6 MIN	5 $\frac{1}{2}$	8 $\frac{1}{2}$	12-16d	8-16d	4467	1430	715
AC6 MAX	5 $\frac{1}{2}$	8 $\frac{1}{2}$	14-16d	14-16d	10000	2500	1070
AC6R MIN	6	9	12-16d	8-16d	4467	1430	715
AC6R MAX	6	9	14-16d	14-16d	10000	2500	1070
ACE6 MIN	—	6 $\frac{1}{2}$	8-16d	6-16d	—	1070	715
ACE6 MAX	—	6 $\frac{1}{2}$	10-16d	10-16d	—	1785	1070
LPC4	3 $\frac{3}{8}$	3 $\frac{1}{2}$	8-10d	8-10d	2333	760	325
LPC6	5 $\frac{1}{8}$	5 $\frac{1}{8}$	8-10d	8-10d	2817	915	490

¹ Allowable loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed; reduce for other load durations according to the code.

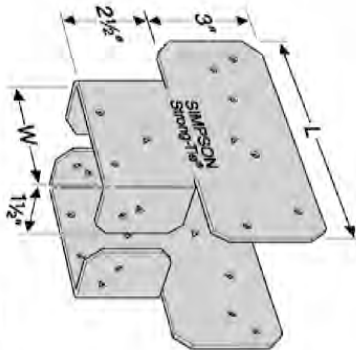
2. Loads apply only when used in pairs.

3. LPC fastener quantities are for two LPCs.

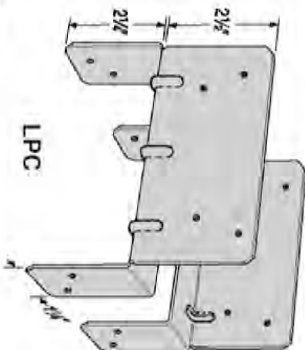
4. LPC lateral load is in the direction of the beam's axis.

5. MIN nailing quantity and load values - fill all round holes.

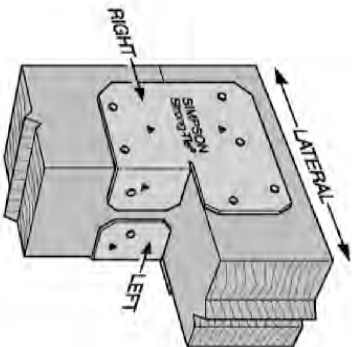
MAX nailing quantities and load values - fill round and triangle holes



AC



LPC



Typical ACE Installation

REV	DATE	DESCRIPTION
PROJECT		
DRAWING DESCRIPTION		
SIMPSON AC/LPC POST CAPS.		
DRAWING NO.		
A-273		
SCALE		
NTS		
CREATION DATE		
5/1/11		
DRAWN BY		
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OBROCK COX		

FACE MOUNT HANGERS
LULUP/U/HU
STANDARD
JOIST HANGERS

See Hanger tables on pages 36 to 39. See Hanger Options on pages 73 to 78 for hanger modifications, which may result in reduced loads.

LU—Value engineered for strength and economy.¹ Just a tap of the hammer at each speed prying secures the LU for easy nailing. Precision-formed—engineered for installation ease and design value.

LUP—Precision-formed and engineered for fast installation and maximum loads. Speed prongs are curved steel for extra strength. One hit positions the hanger for header nailing, eliminating jost nailing and the need for special short nails.

U—The standard U hanger provides flexibility of joist to header installation. Some models have speed prongs for fast installation. Versatile fastener selection with tested allowable loads.

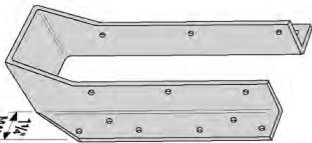
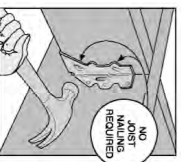
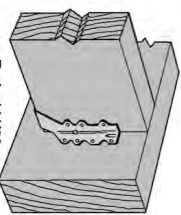
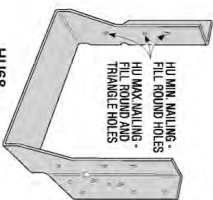
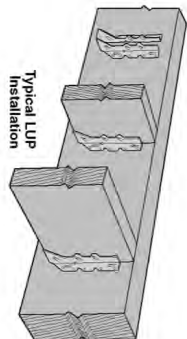
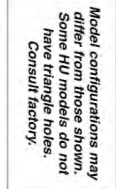
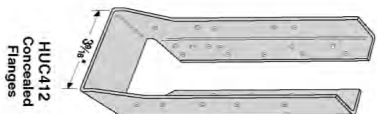
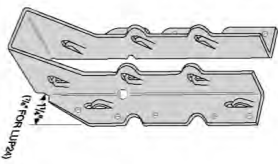
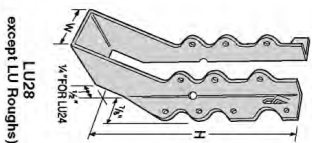
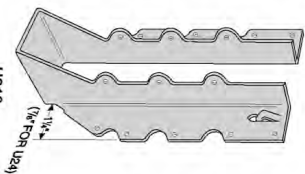
HJ—Most models have triangle and round holes. To achieve maximum loads, fill both round and triangle holes with common nails. These heavy-duty connectors are designed for schools and other structures requiring additional strength, longevity and safety factors.

FINISH: Galvanized

- **U^{UP}**—Speed ponies provide instant location of the hanger onto the carrying beam and harmless securing of the post onto the hanger. If a post prong is detached or bent on a knot, nail a 100x1/4" or equal through that slot. On cantilevered posts, use a 100x1/4" nail at each prong in addition to the prongs.
- **H^U**—can be installed filling round holes only, or filling round and triangle holes for maximum values. See tables.

OPTIONS: • see Hanger Options on page 75, for sloped and/or skewed U/HU models and HUC (concealed hanger) models.
• HU only—rough beam sizes available by special order.

RR 24947, RR 24818. (LUP, U, HU, LU-18 gauge).



HU214
Projection seat on
most models for
maximum bearing and
section economy.

JOIST SHEAR LOADS

The maximum capacity of a horizontal joist or rafter may be limited by the maximum horizontal shear capacity. This table gives the capacity for common sizes

Joist or Rafter	Allowable Shear					
	100		115		125	
	DFL	SPF	DFL	SPF	DFL	SPF
2x4	332	245	382	282	415	306
2x6	522	385	600	443	653	481
2x8	688	508	792	584	860	635
2x10	878	648	1010	745	1098	810

1. DFL = Doug Fir-Larch;
SPF = Spruce-Pine-fir
2. FV = 95 psi for DFL,
70 psi for SPF.
3. The 115 and 125 loads
are 115% and 125%
of the 100% column,
according to the code
for short-term loading.

[illegible]

PROJECT

DRAWING DESCRIPTION

SIMPSON MOUNT HANGERS.

DRAWING NO.

A-274

SCALE

NTS

CREATION DATE

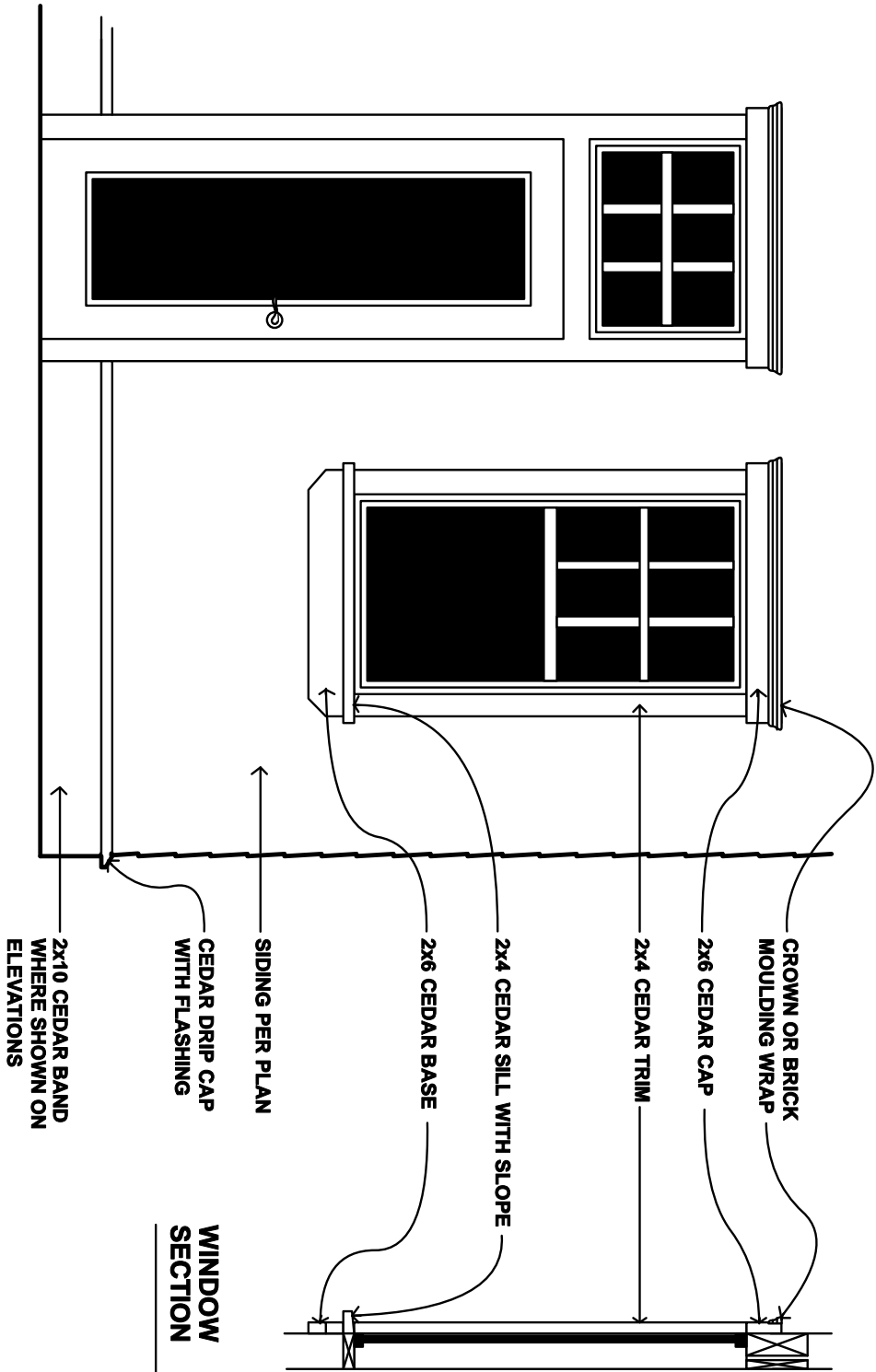
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NOTE: SUGGESTED MATERIAL IS CEDAR BUT PRIMED SPRUCE MAY BE USED AS AN ALTERNATE



EXTERIOR TRIM DETAIL

SCALE: 1/2" = 1'-0"

WINDOW SECTION

REV	DATE	DESCRIPTION

PROJECT

DRAWING DESCRIPTION
EXTERIOR DOOR TRIM DIAGRAM.

DRAWING NO.

A-301

SCALE

NOT TO SCALE

CREATION DATE

5/1/11

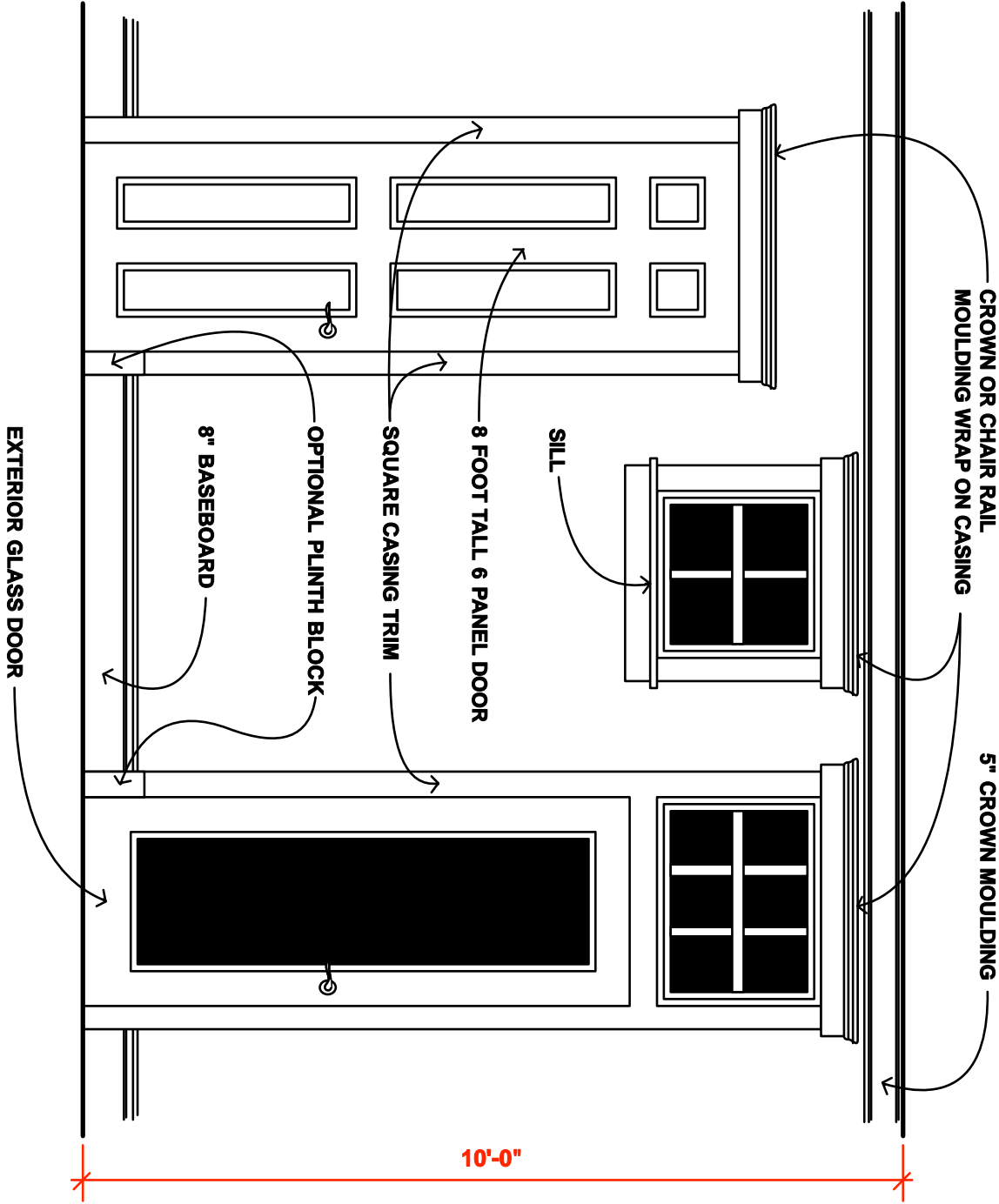
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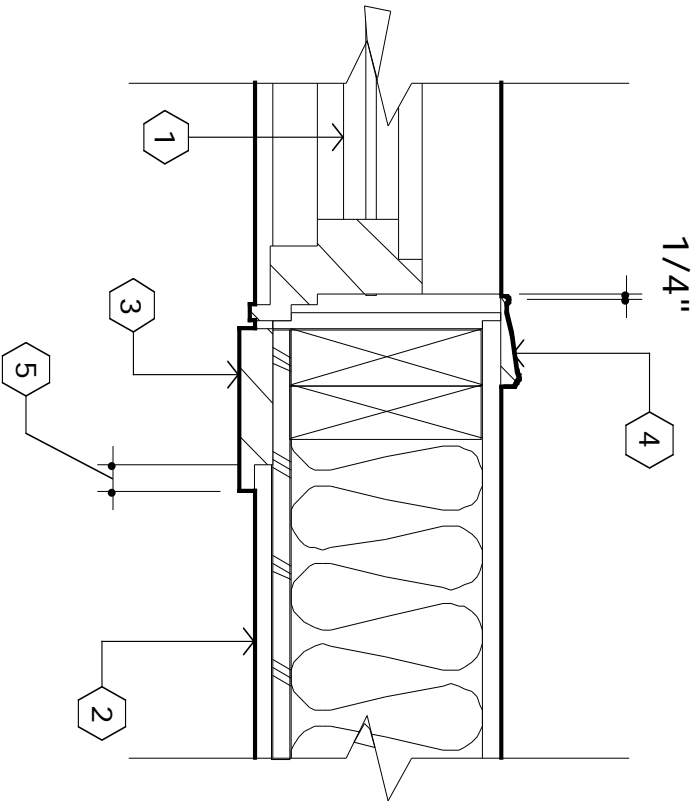
NOTE: ALL TRIM SIZES ARE GENERIC
AND CHOSEN BY AVAILABILITY



INTERIOR TRIM DETAIL FOR 10 FOOT CEILINGS

SCALE: 1/2" = 1'-0"

REV	DATE	DESCRIPTION
PROJECT		
DRAWING DESCRIPTION		
EXTERIOR WINDOW TRIM DIAGRAM.		
DRAWING NO.		
A-302		
SCALE		
1" = 1'-0"		
CREATION DATE		
5/1/11		
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NOTES

- < 1> WINDOW FRAME AND SASH
- < 2> 1/2" SHEATHING
- < 3> 1X4 CEDAR TRIM
- < 4> CASING; REFER TO SPECS.
- < 5> 1/2" DADO TO RECEIVE SIDING
CAULK PRIOR TO INSERTING
SIDING.

REV	DATE	DESCRIPTION
PROJECT		

DRAWING DESCRIPTION
TYPICAL WINDOW JAM SECTION.

DRAWING NO.

A-303

SCALE

NOT TO SCALE

CREATION DATE

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